Chinsegut Hill

ALUMINUM WALKWAY COVER

DESIGN STANDARDS AND LOADS:

DEAD LOAD

THE BUILDING STRUCTURE HAS BEEN DESIGNED USING THE FOLLOWING **DESIGN STANDARDS:**

- 1. 2020 FLORIDA BUILDING CODE (7TH EDITION)
- ASCE 7 MINIMUM DESIGN LOAD FOR STRUCTURES
- SPECIFICATIONS FOR ALUMINUM STRUCTURES. ALUMINUM DESIGN MANUAL, PART 1-A AND PART 1-B OF THE ALUMINUM ASSOCIATION

THE FO	OLLOWING	LOADS	SPECIFICALL	Y DESIGNED	FOR:

ROOF LIVE:	20 PSF
ROOF SNOW LOADING: (NOT GOVERNING)	
P/g:	ZERO PSF
P/f:	ZERO PSF
C/e:	0.9
l/s:	1.00
C/t:	1.0

5 PSF

GEOTECHNICAL DESIGN INFORMATION SOIL DESIGN LOAD BEARING VALUE:

WIND:	
WIND SPEED: VIJIT	140 MPH
V _{ASD}	108 MPH
EXPOSURE CATEGORY:	С
BUILDING RISK CATEGORY:	II
ENCLOSURE CLASSIFICATION:	OPEN
INTERNAL PRESSURE COEFFICIENT:	±0.00
WIND BORNE DEBRIS ZONE:	YES
HIGH VELOCITY WIND ZONE:	NO

^{*} DESIGN IS PREDICATED ON ASD VALUES

COMPONENTS & CLADDING PRESSURES FOR WALKWAY COVERS

Roof Angle	Effective	ASD Wind Pressure p ₂ =q ₂ x G x C/N x 0.6 (psf) (Eq. 30.8-1)					
Roof Aligie	Wind Area	Zone 3		Zone 2		Zone 1	
	≤ 9	60.4	-62.1	45.3	-47.3	30.2	-31.0
≤20°	> 9 ≤ 36	45.3	-47.3	45.3	-47.3	30.2	-31.0
	> 36	30.2	-31.0	30.2	-31.0	30.2	-31.0

DESIGN NOTE: THE WALKWAY COVER SHOWN HEREIN IS NOT CONSIDERED A HABITABLE SPACE. ALL COMPONENTS AND CLADDING ARE SITE SPECIFIC ENGINEERED.

ALUMINUM WALKWAY COVER SYSTEM SHALL BE CONSTRUCTED ENTIRELY OF ALUMINUM EXTRUSIONS. STRUCTURAL FRAMING SHALL CONSIST OF WELDED OR MECHANICALLY CONNECTED ONE-PIECE RIGID STRUCTURAL BENTS (COLUMN AND BEAM ASSEMBLIES), DECKING, ACCESSORY ITEMS AND HARDWARE TO PROVIDE A COMPLETE SYSTEM.

DRAINAGE: WATER FLOW SHALL BE DIRECTED AND DRAIN FROM THE ROOF DECK INTO DESIGNATED BEAMS AND COLUMNS, AND OUT DRAIN HOLES W/ DIVERTERS.

- THE FOLLOWING PUBLICATIONS ARE CONSIDERED A PART OF THIS STRUCTURAL SPECIFICATION:

 1. ALUMINUM DESIGN MANUAL, LATEST EDITION

AWS D1.2: STRUCTURAL WELDING CODE - ALUMINUM

MINIMUM STRENGTH OF MATERIALS (Fv) SHALL BE AS FOLLOWS U.N.O: STRUCTURAL EXTRUSIONS - ASTM B221 6063-T6; 25 KSI ANCHOR BOLTS - ASTM F1554 GR. 36 OR A307; 36 KSI

STRUCTURAL CONNECTION FASTNERS - ASTM A193/A194/F593 TYPE 304, 316, 410, OR 18-8: 30 KSI

ALL ZINC PLATED FASTENERS SHALL BE ISOLATED w/ HARD URETHANE RUBBER GASKETS.

FABRICATOR SHALL FURNISH ALL PLATES, BOLTS AND ANGLES CAST INTO TIE BEAMS, FOOTINGS OR

ALL SHOP CONNECTIONS SHALL BE WELDED AND FIELD CONNECTIONS SHALL BE BOLTED U.N.O.

ALL EXPOSED EDGES AND SURFACES SHALL BE FINISHED AND FREE OF COURSE OR JAGGED EDGES

STRUCTURAL BENTS AND FRAMES

MECHANICAL CONNECTIONS IN BENTS AND/OR FRAMES, IF PRESENT, SHALL CONSIST OF A MINIMUM OF TWO (2) FASTENERS PER CONNECTION U.N.O.

JOINT FASTENERS SHALL BE INSTALLED TO SNUG TIGHT CONDITION U.N.O.

COLUMN SHAPES SHALL BE TUBULAR EXTRUSIONS SIZED FOR STRUCTURAL LOADS. MINIMUM SIZE SHALL BE 6"x6"x0.150"

BEAM SHAPES SHALL BE TUBULAR EXTRUSIONS SIZED FOR STRUCTURAL LOADS. MINIMUM SIZE SHALL RF 6"y6"y0 150"

STRUCTURAL DECK SHALL CONSIST OF INTERLOCKING "SNAP-FIT" ALTERNATING CAP/PAN EXTRUDED COMPONENTS. DECK SYSTEM SHALL CONSIST OF NOMINAL 3" TALL AND NOMINAL 6" INCREMENTAL

STRUCTURAL DECK SECTIONS SHALL BE ATTACHED WITH A MINIMUM OF TWO (2) #14x1" SELF DRILLING SELF TAPPING SS "TEK" SCREWS WITH INTEGRAL NEOPRENE WASHER BENEATH 5/8"Ø CONICAL WASHER.

FASCIA:
END FASCIA SHALL CONSIST OF CUSTOM 3" NOMINAL HEIGHT SECTION FASTENED TO THE INSTALLED

SIDE FASCIA SHALL CONSIST OF 4x6x.093 GUTTER FASCIA EXTRUDED COMPONENTS INSTALLED PER

FASCIA SHALL BE INSTALLED WITH #10 SS SDST "TEK" SCREW AT NOT MORE THAN 36" O.C. UPPER BRACE FASTENER SPACING SHALL NOT EXCEED 72" O.C. & SHALL BE FASTENED w/ (1) #10 SS SDST "TEK" SCREW AT EACH END OF BRACE.

FLASHING REQUIRED AS DICTATED BY PROJECT CONDITIONS SHALL CONSIST OF MIN. 0.040 THICKNESS FLAT MATERIAL CUSTOM FABRICATED AS INDICATED BY THESE DOCUMENTS.

FLASHING SHALL BE FASTENED WITH #10 SS SDST "TEK" SCREWS INTO METALS OR 1/4"x11/4" SS TAPCONS AT A MAXIMUM OF 36" O.C.

FLASHING IN CONTACT WITH UNPAINTED CONCRETE OR DISSIMILAR METALS SHALL BE ISOLATED BY USE OF NEOPRENE STRIP WASHER OR FIELD APPLIED CAULKING

ALL WELDS SHALL BE BY TUNGSTEN INERT GAS (TIG) PROCESS AND PERFORMED BY QUALIFIED

ALL WELDS SHALL COMPLY WITH AWS D1.2

ALL WELDS SHALL BE ARCHITECTURAL GRADE FINISH.

ALL WELDS SHALL BE CONTINUOUS LLN O. WELD SIZE SHALL BE BY AWS STANDARDS LLN O.

SHALL BE VIBROPRUF #110R EQUIVALENT (ONE-PART DRY, NON-SHRINK, NON-METALLIC CEMENTITIOUS MATERIAL)

FINISHES: ALL FINISHES TO BE 204-R1 CLEAR ANODIZE

DIVISION 2 - SITE WORK FOR BUILDING FOUNDATIONS:
THE GENERAL CONTRACTOR SHALL REVIEW THE GEOTECHNICAL REPORT FOR THIS SITE AND SHALL EXCAVATE, FILL, COMPACT AND PREPARE THE EXISTING SOILS AND NEW FILL FOR CONSTRUCTION OF

FOUNDATIONS HAVE BEEN DESIGNED USING 2,000 PSF ALLOWABLE SOIL BEARING

- THE FOLLOWING PUBLICATIONS ARE CONSIDERED A PART OF THIS STRUCTURAL SPECIFICATION:

 1. ACI 318: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
- ACI 308-A: GUIDE TO CONCRETE CURING ACI 360R-08: GUIDE TO DESIGN OF SLABS ON GROUND
- ACI 302.1R-04: GUIDE FOR CONCRETE FLOOR & SLAB CONSTRUCTION

CONCRETE MATERIALS SHALL CONFORM TO ASTM C94 AND AS FOLLOWS:

- PORTLAND CEMENT SHALL CONFORM TO ASTM C150 AND BE TYPE I OR TYPE III.
- AGGREGATES SHALL CONFORM TO ASTM C33, AND SHALL BE WASHED AND CLEAN. WATER SHALL BE CLEAN AND POTABLE
- ADMIXTURES CONTAINING CHLORIDES SHALL NOT BE USED.

MINIMUM CONCRETE COMPRESSION STRENGTH AT 28 DAYS (F'c) SHALL BE AS FOLLOWS:

- 3000 PSI SLAB ON GRADE, TIE BEAMS (3/4" MAX. AGGREGATE)
- 3000 PSI FOOTINGS (1-1/2" MAX. AGGREGATE)

REINFORCEMENT MATERIALS SHALL CONFORM TO THE FOLLOWING:

- BARS SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A 615, GRADE 60.
- WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A 185

MINIMUM REQUIRED CONCRETE COVER FOR REINFORCING STEEL (LINLESS NOTED OTHERWISE):

- FORMED BY EARTH:
- 3" BOTTOM AND SIDES
 - FORMED BY OTHER THAN FARTH & EXPOSED TO FARTH

3" BOTTOM; 2" SIDES (BARS LARGER THAN #5), 1-1/2" SIDES (BARS #5 AND SMALLER)

REINFORCEMENT SHALL BE ACCURATELY PLACED, SUPPORTED, AND SECURED AGAINST DISPLACEMENT. USE METAL CHAIRS, RUNNERS, BOLSTERS, SPACERS, AND HANGERS AS REQUIRED. REINFORCEMENT ACCESSORIES IN CONTACT WITH FORMS SHALL HAVE HOT-DIP GALVANIZED LEGS OR

PROVIDE CORNER BARS AT ALL CORNERS OF FOOTINGS, GRADE BEAMS, EDGE BEAMS, TIE BEAMS AND WALLS. CORNER BARS SHALL BE BENT LAP SPLICE BARS AND SHALL BE THE SAME SIZE AND SPACING AS HORIZONTAL BARS

LAP SPLICES SHALL BE 48 BAR DIAMETERS OR 25 INCHES WHICHEVER IS GREATER

TIE ALL REINFORCING MATS, CAGES, BUNDLES AND OTHER BAR ASSEMBLIES WITH BLACK ANNEALED WIRE, 16 GA MINIMUM.

LAP SPLICES IN HORIZONTAL BARS IN FOOTINGS AND TIE BEAMS SHALL BE STAGGERED. IF LAP SPLICES ARE REQUIRED OVER OPENINGS PROVIDE 48 BAR DIAMETERS MINIMUM

DO NOT WELD REINFORCING BARS. EXCEPT BLITT SPLICES MAY BE WELDED IN ACCORDANCE WITH

ALL FOOTING ELEVATIONS SHOWN ARE TO TOP OF FOOTINGS.

ALL FOOTINGS ARE CENTERED UNDER WALLS OR COLUMNS UNLESS NOTED OTHERWISE

FURNISH CONTINUOUS WALL FOOTING REINFORCING IN STOCK LENGTHS.

PROVIDE DOWELS AND SPLICE BARS AT CORNERS AND THROUGH FOOTING STEPS. CONTINUOUS FOOTING REINFORCING SHALL BE SUPPORTED BY STEEL CHAIRS WITH SAND PLATES OR BY CONCRETE BRICKS. DO NOT USE WIRE SUPPORTS ALONE.

CAST DOWELS IN FOOTINGS FOR CONCRETE WALLS AND COLUMNS ABOVE, WALL DOWELS TO BE SAME NUMBER, SIZE AND SPACING AS THE VERTICAL WALL REINFORCING. COLUMN DOWELS TO BE SAME SIZE AND NUMBER AS VERTICAL COLUMN REINFORCING.

DOWELS IN FOOTINGS ARE TO PROJECT FROM FOOTINGS A MINIMUM OF 25" OR 40 BAR DIAMETERS, WHICHEVER IS GREATER

PROVIDE STANDARD HOOK IN FOOTING DOWELS.

ALL REINFORCEMENT SHALL BE INSTALLED AND SUPPORTED PRIOR TO START OF CONCRETE PLACEMENT, "WET-STICKING" OF REINFORCING IS PROHIBITED

PROVIDE SLEEVES FOR ALL PIPES, DUCTS, CONDUITS, ETC., WHICH PENETRATE CONCRETE STRUCTURAL MEMBERS PRIOR TO PLACEMENT OF CONCRETE. CUTTING OR DRILLING OF HARDENED

CURE ALL CONCRETE IN ACCORDANCE WITH ACI-308.

PROVIDE 3/4" CHAMFERS ON ALL EXPOSED CONCRETE EDGES, UNLESS NOTED OTHERWISE.

APPLY 2-COMPONENT EPOXY RONDING AGENT TO EXISTING SURFACES WHERE NEW CONCRETE IS TO BE PLACED AGAINST EXISTING CONCRETE.

ANCHORING EPOXY SHALL BE 2-PART EPOXY RESIN INJECTION TURE MIXED SLICH AS POWERS. HILTI OR SIMPSON OR ENGINEER APPROVED ALTERNATIVE. INSTALLER SHALL CLOSELY FOLLOW ALL MANUFACTURER INSTALLATION INSTRUCTIONS.



Digitally signed by Ryan Green, P.E.

DN: c=US, st=Florida, l=Fort Myers, o=RLG Engineering, ou=88500, cn=Ryan Green, P.E., email=rlgengineeringllc@gmail.c

Date: 2024.09.21 06:18:11 -04'00'

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED B RYAN L. GREEN ON THE DATE ADJACENT TO THE SEAL

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Aluminum Walkway Cover 09/24/2024 S1 BJE







